

Indexed by

Scopus®

## STRUCTURAL FUZZY RELIABILITY ANALYSIS USING THE CLASSICAL RELIABILITY THEORY



**Tuan-Hung Nguyen**

Thuyloi University, Faculty  
of Civil Engineering, Hanoi,  
Vietnam

**Huynh-Xuan Le**

National University of Civil  
Engineering, Hanoi,  
Vietnam



**Key words:** fuzzy reliability, probability theory, structural reliability, response surface method, energy methods, plastic bending of beams

**doi:**10.5937/jaes0-30656

**Cite article:**

Nguyen T.H., Le H.X. (2021) STRUCTURAL FUZZY RELIABILITY ANALYSIS USING THE CLASSICAL RELIABILITY THEORY, *Journal of Applied Engineering Science*, 19(4), 1074 - 1082, DOI:10.5937/jaes0-30656

**Online access** of full paper is available at: [www.engineeringscience.rs/browse-issues](http://www.engineeringscience.rs/browse-issues)

# STRUCTURAL FUZZY RELIABILITY ANALYSIS USING THE CLASSICAL RELIABILITY THEORY

Tuan-Hung Nguyen<sup>1\*</sup>, Huynh-Xuan Le<sup>2</sup>

<sup>1</sup>Thuyloi University, Faculty of Civil Engineering, Hanoi, Vietnam

<sup>2</sup>National University of Civil Engineering, Hanoi, Vietnam

This study is focused on a novel approach for calculating structural fuzzy reliability by using the classical reliability theory. In order to handle the structural fuzzy reliability problem, the formulae for establishing normal random variables equivalent to symmetric triangular fuzzy number are presented. From these equivalent random ones, the original problem is converted to the basic structural reliability problems, then the methods of the classical reliability theory should be applied to calculate. Moreover, this study proposes two notions in terms of central fuzzy reliability and standard deviation of fuzzy reliability as well as a calculation procedure to define them. Lastly, the ultimate fuzzy reliability of the proposed method is established and utilized to compare the allowable reliability in the design codes. Numerical results are supervised to verify the accuracy of the proposed method.

**Key words:** fuzzy reliability, probability theory, structural reliability, response surface method, energy methods, plastic bending of beams

## INTRODUCTION

In engineering structures, most of the input data, such as load characteristics, material properties, boundary conditions, geometric dimensions, load-carrying capacities, contain non-deterministic quantities, which are described as uncertainty variables. In [1], uncertainties present in a structural system can be categorized as either aleatory or epistemic. If the input data are random parameters, the limit state function is a random parameter, is expressed as follows

$$M=R-S \tag{1}$$

where M is the limit state function;  
R is the resistance function;  
S is the load effect function.

The structural reliability is defined as follows [2, 3]

$$P_s = Prob (M>0) \tag{2}$$

Due to using the probability theory, which is the most complete theory, assessing structures by the reliability index is prescribed in the structural design code [4]. When the input data in structural systems are epistemic uncertainties, depending on how to describe the uncertainties, the structural reliability can be derived by the different approaches, as using either the fuzzy sets theory

[5-12] or the fuzzy random theory [13,14]. In this study, we only consider that the epistemic ones are represented as fuzzy numbers, which are always interested in the reality problems. In this case, the structural reliability had the different names, such as the safety possibility of structures [6], the fuzzy reliability index [7, 8], the fuzzy reliability [9, 10, 11]. The terminologies are named the fuzzy reliability later. This fuzzy reliability can be classified into three classes: the resistance function R and the load effect S are fuzzy numbers, the resistance R is a random parameter and the load effect S is a fuzzy number, the resistance R is a fuzzy number and the load effect S is a random parameter. These approaches [5-12] for the fuzzy reliability are detailed analyzed below.

According to Dong et al. [5], the fuzzy failure is determined based on the two  $\alpha$ -cuts  $M_\alpha$  and  $R_\alpha$  (Fig.1a) as follows

$$FP = \frac{1}{2} [T(\tilde{S} > \tilde{R}) + T(\tilde{S} > \tilde{R})] \tag{3}$$

In Fig. 1a, the fuzzy failure (FF) and the fuzzy reliability (FR) are expressed as follows, respectively

$$FF = \frac{h}{2}, FR = 1 - \frac{h}{2} \tag{4}$$

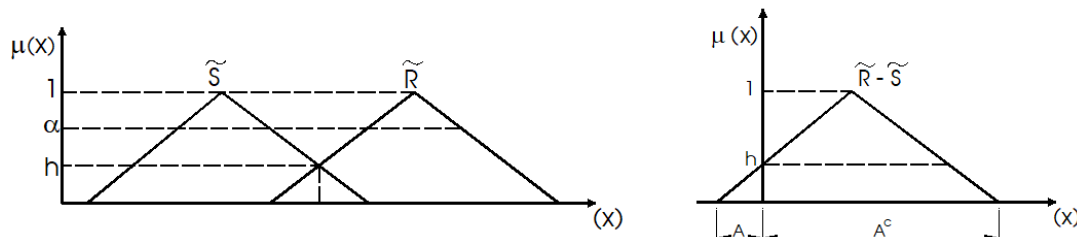


Figure 1: The method [5]

- a. The interference model of the fuzzy resistance number  $\tilde{R}$  and the fuzzy load effect number  $\tilde{S}$
- b. The subtraction of the fuzzy resistance number  $\tilde{R}$  and the fuzzy load effect number  $\tilde{S}$

From formula (4), one realizes that the fuzzy failure is the average of the possibility measure and the necessity measure of event A (Fig. 1b) according to the possibility theory [15]. Hence, the method [6] is an approximation of the average, and the transformation from logical expression (3) to formula (4) is an intuitive formula. Besides, the effect of the spread of fuzzy numbers  $R^-$  and  $S^-$  aren't considered absolutely.

Differed from Dong et al. [5], Sherstha and Duckstein [6] considered directly  $\tilde{M}$ , which is the subtraction of the fuzzy resistance number and the fuzzy load effect number, and the fuzzy reliability is defined as the ratio of the membership function of  $\tilde{M}$  which is greater than 0 to the total area of the membership function of  $\tilde{M}$  (Fig. 2)

$$FR = \frac{\int_{M>0} \mu_M(x) dx}{\int \mu_M(x) dx} \quad (5)$$

where  $\mu_M(x)$  is the membership function of fuzzy number  $M^-$

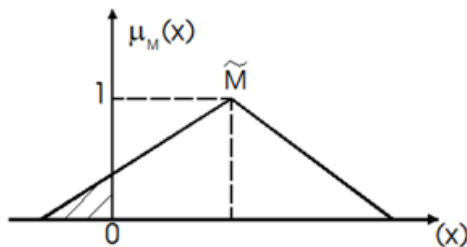


Figure 2: The method [6, 7]

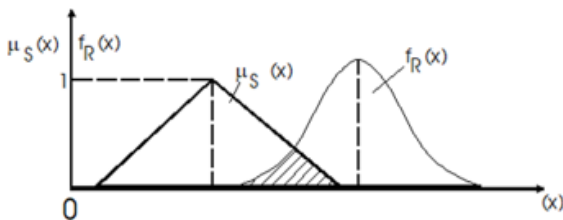


Figure 3: The interference model [10, 11]

The formula (5) is appropriate for the geometrical definition of probability in [16]. Nevertheless, it should be realized that the membership functions in the fuzzy sets theory and the density distribution functions in the probability theory are not equivalent representations of uncertainty. Hence, the fuzzy reliability in the formula (5) is only similar to the probabilistic reliability in the classical reliability theory. Based on the formula (5), Park et al. [7] determined the fuzzy reliability when the membership function  $\mu_M(x)$  is triangular and trapezoidal. Rezaei et al. [8] extended formula (5) in case fuzzy variables formed the pyramid to assess fuzzy reliability of the wing flutter speed. In [9], Li et al. proposed a formula for fuzzy reliability analysis of mechanical structures when the stress  $S$  was modeled as a fuzzy number with given membership function  $\mu_S(x)$  and the strength  $R$  was modeled as a random variable with given distribution function  $f_R(x)$  (Fig. 3). The authors idealized that fuzzy reliability in the fuzzy stress - random strength interference model was a real value, and the Zadeh's notion for the probability of a

fuzzy event [17] is used to calculate. The fuzzy reliability is given as follows

$$FR = 1 - FF = 1 - \int \mu_S(x) f_R(x) dx \quad (6)$$

The drawback of this method is that two functions are under integral in the formula (6) have different measurements, the area between the curve  $f_R(x)$  and the abscissa is unit but for the fuzzy number  $\mu_S(x)$  isn't. In order to overcome this drawback, Jiang and Chen [10] supposed that the  $\alpha$ -cuts of a fuzzy number were the linear distributions, then computed the conventional probability at various  $\alpha$ -cuts and the fuzzy reliability was the average of the reliabilities at the  $\alpha$ -cuts. Tang et al. [11] also utilized this assumption and computed the fuzzy failure probability by applying the Gauss-Legendre quadrature formula. However, it should be seen that different distribution assumptions of the  $\alpha$ -cuts lead to totally different fuzzy reliability results.

From the above analysis, one realizes that it had diversity approaches for evaluating the structural fuzzy reliability. Nevertheless, the meaning of the existing formulas is less evident than the reliability in the classical reliability theory. In order to overcome this drawback, using the traditional reliability analysis theory as a basis for assessing fuzzy reliability is a reasonable approach, due to the fact that the traditional probabilistic methods remain dominant in the field of measurements [18] and are well established in the decision making problems [19]. Hence, the aim of this study is to establish a novel approach for calculating structural fuzzy reliability by using the classical reliability theory. Firstly, the transformation from fuzzy number to equivalent normal random variables is presented. As a consequence, a fuzzy reliability problem is replaced by the traditional reliability problems, and the classical methods could be utilized to solve. Additionally, two notions including the central fuzzy reliability and the standard deviation of fuzzy reliability are proposed. While the central fuzzy reliability is the mean value of fuzzy reliability, the remain notion represents a measure of the spread of fuzzy reliability around its own central. Lastly, the ultimate fuzzy reliability is constituted based on three sigma rule, and is easily compared with the allowable reliability in the structural design codes. Numerical results are thoroughly explored to demonstrate the accuracy of the proposed method.

## METHODS

### The formula for computing the deviation of the equivalent normal random variable

The transformation from fuzzy numbers into random quantities and conversely should be taken into account in any problem where heterogeneous uncertain and imprecise data appear together (e.g. information deficit, linguistic variables, statistical data). The representative transformation principles, such as insufficient reason, maximum specify, uncertainty invariance, are proposed by Dubois et al. [18, 20], Dubois [21] and Klir [15]. Based

on combining the principles of insufficient reason and maximum specificity, Tuan and Huynh [22] proposed an innovation transformation to calculate the deviation of the equivalent normal random variable. For the transformation from standardized symmetric triangular fuzzy number (Fig. 4) into random quantity, the error of probability measure between equivalent probability density function  $p(x)$  of standardized symmetric triangular fuzzy number got by the principle of insufficient reason and normal random variable  $p_1(x)$  is expressed by the following formula

$$(P(A)-P_1(A))^2 \rightarrow \min \quad \forall x \in [-1,0] \quad (7)$$

where  $p(x)$ ,  $p_1(x)$ ,  $P(A)$ ,  $P_1(A)$  are determined as follows

$$p(x) = \begin{cases} -\frac{1}{2} \ln(-x) & ; x \in [-1,0) \\ -\frac{1}{2} \ln(x) & ; x \in (0,1] \end{cases} \quad (8a)$$

$$p_1(x) = \frac{1}{\sqrt{2\pi}\sigma} e^{-\frac{x^2}{2\sigma^2}} \quad (8b)$$

$$P(A) = \frac{1}{2} [x - x \ln(-x) + 1] \quad (8c)$$

$$P_1(A) = \int_{-1}^x \frac{1}{\sqrt{2\pi}\sigma} e^{-\frac{y^2}{2\sigma^2}} dy \quad (8d)$$

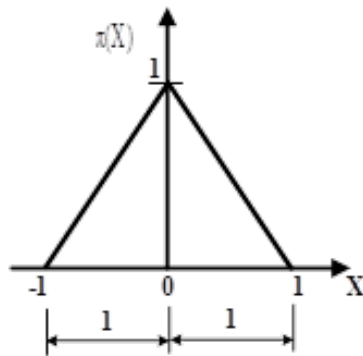


Figure 4: Standardized fuzzy variable

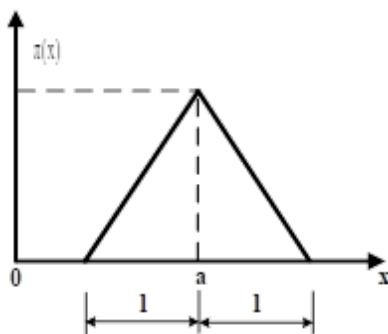


Figure 5: Symmetric triangular fuzzy variable

From (7) we get

$$F_1(\sigma) = \int_{-1}^0 (P(A)-P_1(A))^2 dx \rightarrow \min \quad (9)$$

Due to the different domain between probability density function  $p(x)$  and normal random variable  $p_1(x)$ , in order that probability of density function  $p_1(x)$  in  $(-\infty, 1)$  be in-

significant, one needs

$$F_2(\sigma) = P_1[A^*:x_0 \in (-\infty, -1)] = \int_{-\infty}^{-1} \frac{1}{\sqrt{2\pi}\sigma} e^{-\frac{x^2}{2\sigma^2}} dx \rightarrow \min \quad (10)$$

Combine (9) and (10), we have

$$F(\sigma) = F_1(\sigma) + F_2(\sigma) = \int_{-1}^0 (P(A)-P_1(A))^2 dx + \int_{-\infty}^{-1} \frac{1}{\sqrt{2\pi}\sigma} e^{-\frac{x^2}{2\sigma^2}} dx \rightarrow \min \quad (11)$$

For the reverse transformation from normal random variable into equivalent fuzzy number, the error of possibility measure between equivalent fuzzy number of normal random variable got by the principle of maximum specificity and standardized fuzzy number is expressed by the following formula

$$G(\sigma) = \int_{-1}^0 (\pi_1(x)-1-x)^2 dx + \int_{-\infty}^{-1} \pi_1^2(x) dx \rightarrow \min \quad (12)$$

where  $\pi_1(x)$  is the equivalent fuzzy number of normal random variable determined as follows

$$\pi_1(x) = \pi_1(-x) = \int_{-6\sigma}^x p_1(y) dy + \int_{-x}^{6\sigma} p_1(y) dy \quad (13)$$

In order to solve the multiobjective optimization problem (11) and (12), transforms multiple objectives into a scalar objective function by multiplying each objective function by a weighting factor and summing up all contributors

$$H(s) = \gamma F(s) + (1-\gamma)G(s) \rightarrow \min \quad (14)$$

where  $\gamma \in [0, 1]$ .

For the mathematical meaning, formula (14) is an extension which modifies the equivalent characteristic according to two principles: the principle of insufficient reason when going from fuzzy number to random variable, and the principle of maximum specificity when going from random variable to fuzzy number. For solving (14), Genetic algorithm (GA) [23] is applied using the built-in functions in Matlab. The relation between weighting factor  $\gamma$  and deviation  $\sigma$  is detailed depicted in Fig. 6.

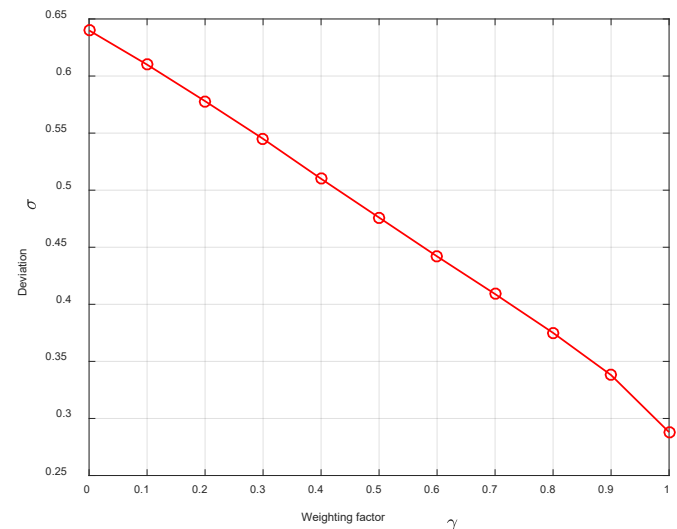


Figure 6: Representation of the relation between deviation and weighting factor

One realizes that the proposed transformation encodes a family of normal random variables including the result of Klir's method, from the standardized fuzzy number. Indeed, the deviation of normal random variable using uncertainty invariance principle is of 0.3989 [24], as the same as the result attains according to the proposed transformation when weighting factor is of 0.728. Hence, the constraints generated in the proposed transformation are more flexible than Klir's method.

For a symmetric triangular fuzzy number  $\tilde{X}=(a,l)_{LR}$  (Fig. 6), the relation between it and the standardized fuzzy variable  $\tilde{x}=(0,1)_{LR}$  is defined as follows [22]

$$\tilde{x} = \frac{\tilde{X}-a}{l} \quad (15)$$

### The central fuzzy reliability, the standard deviation of fuzzy reliability

As a result of accomplishing a family of normal random variables, the original structural fuzzy reliability problem is converted to the basic reliability problems using the classical reliability theory to solve. By this we mean that the calculated reliability will vary from value to value. Therefore, the attained fuzzy reliability can be treat as a random variable with its own mean, standard deviation.

Based on the relation between the deviation of equivalent normal random variable and weighting factor explored in the previous section, three especial values of the weighting factor  $\gamma$  are considered:

$$\text{When } \gamma = 0.5 \text{ we get } \sigma = 0.476. \quad (16.a)$$

$$\text{When } \gamma = 1 \text{ we get } \sigma = 0.288. \quad (16.b)$$

$$\text{When } \gamma = 0 \text{ we get } \sigma = 0.640. \quad (16.c)$$

The value  $\sigma = 0.476$  with  $\gamma = 0.5$  in formula (16.a) means to choice the equilibrium for the weighting factors of objective function  $F(\sigma)$  and objective function  $G(\sigma)$ . Due to this reason, the value  $\sigma = 0.476$  is utilized to calculate the central fuzzy reliability  $FR_c$  being the mean value of fuzzy reliability. Two remain values in formula (16.b) and (16.c) are the minimum and the maximum values of the deviation of equivalent normal random variable, respectively. Therefore, we will utilize them to calculate the standard deviation of fuzzy reliability  $\sigma_{FR}$ .

Following that, without loss of generality, one considers the limit state function is given by

$$g(x)=R-S=g(x_F, x_R)=g(\tilde{x}_1, \tilde{x}_2, \dots, \tilde{x}_n, x_r) \quad (17)$$

where  $x_F=(\tilde{x}_1, \tilde{x}_2, \dots, \tilde{x}_n)$  are independent fuzzy variables, as symmetric triangular fuzzy number  $\tilde{x}_i=(a_i, l_i)_{LR}$ ;  $x_R$  is random parameter of the resistance function or the load effect.

In order to determine reliability according to the traditional reliability theory, the transformation from fuzzy variable  $\tilde{x}_i=(a_i, l_i)_{LR}$  to normal random variable  $x_i \sim N(\mu_i, \sigma_i)$  is accomplished based on formulas (15) and (16), it means:

$$\text{When } \gamma = 0.5 : \mu_i = a_i, \sigma_i = 0.476 l_i \quad (18.a)$$

$$\text{When } \gamma = 1.0 : \mu_i = a_i, \sigma_i = 0.288 l_i \quad (18.b)$$

$$\text{When } \gamma = 0.0 : \mu_i = a_i, \sigma_i = 0.640 l_i \quad (18.c)$$

For calculating the central fuzzy reliability  $FR_c$ , the mean and the deviation of the equivalent normal random variable determined by formula (18.a) are applied. Then, can utilize any techniques in the classical reliability theory, such as First-order reliability method (FORM), Second-order reliability method (SORM), Cornell reliability index, Hasofer-Lind reliability index, Monte Carlo method, so on, to estimate structural reliability.

Due to the fact that a large amount of the means and the deviations of the equivalent normal random variables are determined by solving the formulae (14) with weighting factor  $\gamma$  is fixed, there should be accomplished a large number of problems to determine the standard deviation  $\sigma_{FR}$  of fuzzy reliability. In order to reduce the number of computations, the central composite design in the response surface method [25] is applied and the standard deviation of fuzzy reliability is calculated as follows

$$\sigma_{FR} = \sqrt{\frac{\sum_{k=1}^m (FR_{s_k} - FR_c)^2}{(m-1)}} \quad (19)$$

where  $FR_{s_k}$  is the reliability value at the  $k$ th experimental of the central composite design, with its own mean, standard deviation are defined by formulas (18.b) and (18.c);  $FR_c$  is the central fuzzy reliability;

$m$  is the total number of test runs in the central composite design:  $m = 2^n + 2n$ , with  $n$  is the number of fuzzy variables.

### The ultimate fuzzy reliability

In order to take into account the bias of fuzzy reliability, the ultimate fuzzy reliability  $FR_u$  is presented and used to compare the allowable reliability in the structural design codes. Based on three sigma rule in the probability theory, the ultimate fuzzy reliability is computed as follows

$$FR_u = FR_c - 3\sigma_{FR} \quad (20)$$

When the input data has only one fuzzy variable ( $n=1$ ), the ultimate fuzzy reliability is calculated as follows

$$FR_u = \min(FR_1, FR_2) \quad (21)$$

where  $FR_1 = P_s(x_1, x_R)$ ,  $FR_2 = P_s(x_2, x_R)$  with  $x_1$  and  $x_2$  are equivalent normal random variables, which have mean  $\mu_i$  and deviation  $\sigma_i$  is defined by formulas (18.b) and (18.c), respectively.

## NUMERICAL EXAMPLES AND DISCUSSIONS

In order to verify the proposed method, the illustrative examples including mathematical examples and engineering examples are given. The limit state functions in the illustrative examples are explicit functions, in order to get to a resemblance to structural analysis methods.

### Example 1

Hypothetical limit state function with three variables is indicated as follows

$$g(x) = 2.0 - 0.32(x_1 - 1)^2 x_2^2 - x_2 + x_3^3 - 0.2 \sin(x_1, x_3) \quad (22)$$

where  $x_1, x_2, x_3$  are assumed to be symmetric triangular fuzzy numbers  $(0, 1)_{LR}$ .

The fuzzy reliability FR of the method [5] is:  $FR_D = 0.89053$ .

The fuzzy reliability FR of the method [6] is:  $FR_S = 0.918638$ .

The relative difference  $\epsilon$  between  $FR_D$  and  $FR_S$  is:

$$\epsilon = 100 \cdot \frac{(FR_D - FR_S)}{FR_S} = -3.05293\% \quad (23)$$

The Monte Carlo method [2, 3] with a number of trials  $N_s = 106$  is used to calculate in the proposed method. Tab. 1 displays the results of the central and the ultimate fuzzy reliability of the proposed method and the relative difference  $\epsilon$  of the fuzzy reliability between the proposed method and the method [6].

Table 1: The reliability of the proposed method and the method [6]

The fuzzy reliability	The fuzzy reliability FR of the proposed method	The fuzzy reliability $FR_S$ of the method [6]	The relative difference $\epsilon$ of the fuzzy reliability FR
$FR_c$	0.989040	0.918638	7.66371
$FR_u$	0.936526	0.918638	1.94726

### Example 2

Consider a two-story frame structural system in Fig. 7, two girders are infinitely rigid. Elastic modulus E, ceiling height H, loads  $P_1$  and  $P_2$  are assumed to be symmetric triangular fuzzy numbers:  $\tilde{E} = (2 \times 10^7, 1.2 \times 10^5)_{LR}$  (unit: kN/m<sup>2</sup>);  $\tilde{H} = (3, 0, 2)_{LR}$  (unit: m);  $\tilde{P}_1 = (20, 2)_{LR}$  (unit: kN). The width b and the height h of the column are certain variables with 0.2 m and 0.3m, respectively.

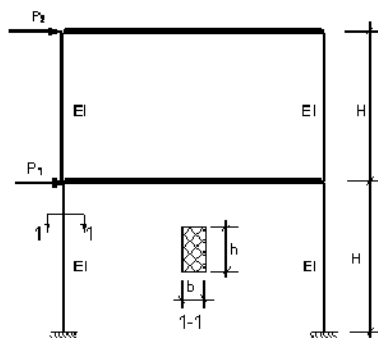


Figure 7: Two-story frame structural system

Applying the shear forces distribution method, horizontal displacement at the top of the structural system is expressed as follows

$$\Delta_t = \frac{(2P_2 + P_1)H^3}{24EI} \quad (24)$$

where  $I = bh^3/12 = 4.5 \times 10^{-4} \text{ m}^4$ .

Because the prescribed horizontal displacement at the top

of frame structural is  $2H/500$ , the limit state function in terms of horizontal displacement at the top is given as follows

$$g(x) = \frac{H}{250} - \frac{(2P_2 + P_1)H^3}{24EI} \quad (25)$$

The fuzzy reliability FR of the method [5] is:  $FR_D = 0.892432$ .

The fuzzy reliability FR of the method [6] is:  $FR_S = 0.970360$ .

The relative difference  $\epsilon$  between  $FR_S$  and  $FR_D$  is:

$$\epsilon = 100 \cdot \frac{(FR_D - FR_S)}{FR_S} = -8.03085\% \quad (26)$$

The second-order reliability method SORM [27, 28] is used to calculate the central and the ultimate fuzzy reliability in the proposed method. The derived results are compared with the method [6] and represented in Tab. 2.

Table 2: The results of the proposed method and that of the method [6]

The fuzzy reliability FR	The proposed method	The fuzzy reliability FRs according to the method [6]	The relative difference $\epsilon$ of the fuzzy reliability FR
$FR_c$	0.994769	0.970360	2.51548
$FR_u$	0.955102	0.970360	-1.57237

### Example 3

Determine the fuzzy reliability in terms of plastic collapse of the steel beam is shown in Fig. 8. Two cases are considered as follows:

Case 1: The length L, the width b and the height h of the steel beam are certain variables with 4m, 4 cm and 8 cm, respectively. The concentrated load  $\tilde{P}$  (unit: kN) and the uniform load  $\tilde{q}$  (unit: kN/m) are assumed as symmetric triangular fuzzy numbers given as following:  $\tilde{P} = (10, 2)_{LR}$ ,  $\tilde{q} = (8, 1)_{LR}$ . The yield stress is considered as a normal distribution with the mean value of 24 kN/cm<sup>2</sup> and the standard deviation of 2 kN/cm<sup>2</sup>.

Case 2: The length  $\tilde{L}$  (unit:m), the width  $\tilde{b}$  and the height  $\tilde{h}$  of the steel beam (unit: cm), the concentrated load  $\tilde{P}$  (unit: kN), the uniform load q (unit: kN/m) and the yield stress  $\tilde{\sigma}_y$  (unit: kN/cm<sup>2</sup>) are assumed as symmetric triangular fuzzy numbers given as following:  $\tilde{L} = (4, 0.5)_{LR}$ ,  $\tilde{b} = (4, 1)_{LR}$ ,  $\tilde{h} = (8, 1)_{LR}$ ,  $\tilde{P} = (15, 3)_{LR}$ ,  $\tilde{q} = (10, 2)_{LR}$ ,  $\tilde{\sigma}_y = (24, 2)_{LR}$ .

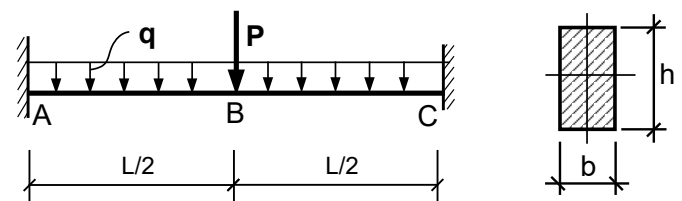


Figure 8: The built-in steel beam fixed at both ends

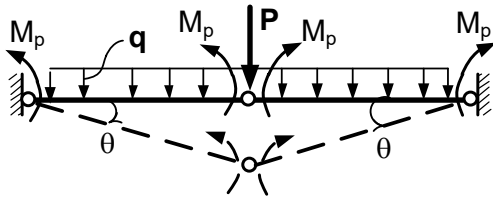


Figure 9: Plastic collapse of the steel beam fixed at both ends

The steel beam is statically indeterminate to the second degree, therefore three hinges are required to change it from a girder structure to a mechanism, as shown in Fig. 9.

Then, the principle of virtual work is applied to determine the limit state function in terms of plastic collapse.

Work done by three hinges during collapse

$$A = M_p \cdot \theta + M_p \cdot 2\theta + M_p \cdot \theta = 4M_p \cdot \theta \quad (27)$$

Work done by the concentrated load  $P$  and the uniform load  $q$

$$T = qL \cdot \frac{L}{4} \cdot \theta + P \cdot \frac{L}{2} \cdot \theta \quad (28)$$

$$\text{Equating (27) and (28): } M_p = \frac{qL^2}{16} + \frac{PL}{8} \quad (29)$$

where  $M_p = \sigma_y \cdot W_p$ , with  $W_p$  - the plastic moment of resistance. Hence, the limit state function in terms of plastic collapse is given as follows

$$g(x) = \sigma_y - \frac{qL^2}{16 \cdot W_p} - \frac{PL}{8 \cdot W_p} \quad (30)$$

$$\text{with the solid rectangular section: } W_p = \frac{bh^2}{4}$$

Case 1:

- The fuzzy reliability of the method [9] is:  $FR_L = 0.840945$ .
- The fuzzy reliability FR of the method [10] is:  $FR_J = 0.958025$ .
- The relative difference  $\varepsilon$  between  $FR_L$  and  $FR_J$  is:

$$\varepsilon = 100 \cdot \frac{(FR_L - FR_J)}{FR_J} = -12.22093\% \quad (31)$$

Case 2:

- The fuzzy reliability of the method [5] is:  $FR_D = 0.693371$ .
- The fuzzy reliability FR of the method [6] is:  $FR_S = 0.792823$
- The relative difference  $\varepsilon$  between  $FR_D$  and  $FR_S$  is:

$$\varepsilon = 100 \cdot \frac{(FR_D - FR_S)}{FR_S} = -12.54398\% \quad (32)$$

Tab. 3 and Tab. 4 display the results of the central and the ultimate fuzzy reliability of the proposed method and the relative difference  $\varepsilon$  of the fuzzy reliability between

the proposed method and the method [10] in Case 1 and the method [6] in Case 2. The Cornell reliability index is applied in terms of Case 1 and the FORM is applied in terms of Case 2 in the proposed method.

Table 3: The results of the proposed method and that of the method [10] in Case 1

The fuzzy reliability FR	The proposed method	The fuzzy reliability $FR_J$ according to the method [10]	The relative difference $\varepsilon$ of the fuzzy reliability FR
$FR_c$	0.948643	0.958025	-0.97927
$FR_u$	0.9221510	0.958025	-3.74460

Table 4: The results of the proposed method and that of the method [6] in Case 2

The fuzzy reliability FR	The proposed method	The fuzzy reliability $FR_S$ according to the method [6]	The relative difference $\varepsilon$ of the fuzzy reliability FR
$FR_c$	0.934335	0.792823	17.84907
$FR_u$	0.794347	0.792823	0.19218

#### Example 4

Determine reliability in terms of stability of the structural system is shown in Fig.10. Two cases are considered as follows:

Case 1: Inertia moment  $I = 808\text{cm}^4$ , length  $l = 5\text{m}$ . Elastic modulus  $E$  is considered as a normal distribution with the mean value of  $2.1 \times 10^4 \text{ kN/cm}^2$  and the standard deviation of  $2.5 \times 10^3 \text{ kN/cm}^2$ . Concentrated load  $P$  (units:kN) is considered as symmetric triangular fuzzy numbers given as following:  $\tilde{P} = (110, 10)_{LR}$ .

Case 2: Inertia moment  $I = 808\text{cm}^4$ . Length  $l$  (units: m), elastic modulus  $E$  (units:  $\text{kN/cm}^2$ ) and concentrated load  $P$  (units:kN) are assumed as symmetric triangular fuzzy numbers given as following:  $\tilde{l} = (5, 0.5)_{LR}$ ,  $\tilde{E} = (2.1 \times 10^4, 2.5 \times 10^3)_{LR}$ ,  $\tilde{P} = (110, 10)_{LR}$ .

In order to calculate in terms of stability, the structural system is transformed to the continuous beam of two equal spans in Fig. 11. The stiffness  $k$  of the intermediate elastic support is determined by bellow formula

$$\frac{1}{k} = \frac{1}{EA} \left/ + \frac{l^3}{48 \cdot (2EI)} = \frac{49l^3}{96EI} \Rightarrow k = \frac{96EI}{49l^3} \quad (33)$$

According to formula (33), it is evident that the value of  $k < 16\pi^2 EI / (2l)^3$  corresponds to stability loss in the symmetrical mode.

The equation in terms of stability is represented in the following form [26]

$$\eta_1(v) = \frac{k \cdot (2l)^3}{48EI} - \frac{16}{49} \quad (34)$$

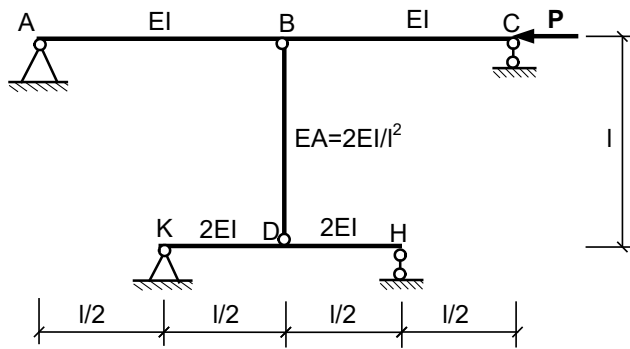


Figure 10: The structural system

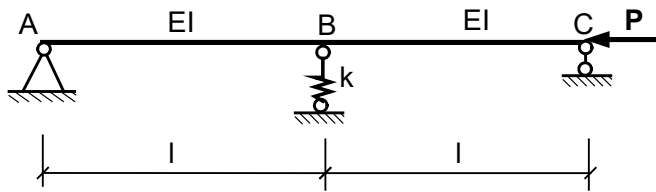


Figure 11: The diagram for calculating stability

Solve the equation (34) obtained  $v=1.80475$ .

The critical load is

$$P_{cr} = v^2 \frac{EI}{l^2} = 1.80475 \frac{EI}{l^2} \quad (35)$$

The limit state function in terms of stability is given as follows

$$g(x) = 1.80475 \frac{EI}{l^2} - P \quad (36)$$

Case 1:

- The fuzzy reliability of the method [9] is:  $FR_L = 0.812516$ .
- The fuzzy reliability FR of the method [10] is:  $FR_J = 0.800996$ .
- The relative difference  $\epsilon$  between  $FR_L$  and  $FR_J$  is:

$$\epsilon = 100 \cdot \frac{(FR_L - FR_J)}{FR_J} = 1.43826\% \quad (37)$$

Case 2:

- The fuzzy reliability of the method [5] is:  $FR_D = 0.698673$ .
- The fuzzy reliability FR of the method [6] is:  $FR_S = 0.828759$ .
- The relative difference  $\epsilon$  between  $FR_D$  and  $FR_S$  is:

$$\epsilon = 100 \cdot \frac{(FR_D - FR_S)}{FR_S} = -15.6964\% \quad (38)$$

Tab. 5 and Tab. 6 display the results of the central and the ultimate fuzzy reliability of the proposed method and the relative difference  $\epsilon$  of the fuzzy reliability between the proposed method and the method [10] in Case 1 and the method [6] in Case 2. The Cornell reliability index is applied in terms of Case 1 and the SORM is applied in terms of Case 2 in the proposed method.

Table 5: The results of the proposed method and that of the method [10] in Case 1

The fuzzy reliability FR	The proposed method	The fuzzy reliability $FR_J$ according to the method [10]	The relative difference $\epsilon$ of the fuzzy reliability FR
$FR_c$	0.792281	0.800996	-1.08795
$FR_u$	0.783605	0.800996	-2.17113

Table 6: The results of the proposed method and that of the method [6] in Case 2

The fuzzy reliability FR	The proposed method	The fuzzy reliability $FR_S$ according to the method [6]	The relative difference $\epsilon$ of the fuzzy reliability FR
$FR_c$	0.924816	0.828759	11.58050
$FR_u$	0.814419	0.828759	-1.73027

### Example 5

In order to verify design bearing capacity of the prestressed concrete pile with the cross-sectional area (40x40) cm, six testing piles under static axial compressive loads have been proceed. The allowable loads calculated from the test results based on Vietnamese national standard TCVN 9393:2012 [29] are displayed in Tab. 7. The design load  $P_d$  is of 1400 kN.

Requires: Determine the safety level in terms of the design bearing capacity of the pile.

Table 7: Summary of test pile properties

Pile/Test No.	Embedded Length L (m)	The failure load (kN)	The allowable load $P_a$ (kN)
1	23.2	3600	1800
2	22.8	3600	1800
3	21.5	2700	1350
4	21.2	2700	1350
5	22.3	3150	1575
6	22.5	3150	1575
Mean value	22.25	3150	1575

Based on the data obtained from Tab.7, one realizes that all numbers are equally likely to appear. Due to the fact that only a small number of observations are available, so as to determine safety level in terms of the design bearing capacity of the pile based on the classical approach, the allowable load  $P_a$  need to be supposed as a uniform distribution within a range [1350, 1800] kN.

As a results, the reliability in terms of the design bearing capacity of the pile is given as follows



$$P_s = 1 - \frac{(1400-1350)}{(1800-1350)} = 0.888889 \quad (39)$$

In the proposed method, triangular membership function can be assigned to the allowable load  $\tilde{P}_a$ , with the belief value of the membership function equals to the mean value, and the zero  $\alpha$ -cut of fuzzy number is created by the extreme values [1350, 1800] kN. Therefore, the allowable load  $\tilde{P}_a$  is the symmetric triangular fuzzy number:  $\tilde{P}_a = [1575, 225]_{LR}$  (unit: kN).

Then, the limit state function in terms of the design bearing capacity of the pile is given as follows

$$g(x) = \tilde{P}_a - P_d \quad (40)$$

Tab. 8 displays the results of the central and the ultimate fuzzy reliability of the proposed method and the relative difference  $\epsilon$  of the fuzzy reliability between the proposed method and the classical reliability method based on assumption for distribution of uncertain variable.

*Table 8: The results of the proposed method and that of the classical reliability method based on assumption for distribution of uncertain variable*

The fuzzy reliability FR	The proposed method	The classical reliability method $P_s$	The relative difference $\epsilon$ of the fuzzy reliability FR
FRc	0.948869	0.888889	6.74779
FRu	0.887870	0.888889	-0.11463

## DISCUSSIONS

Analysis the results of the above examples, the following discussions are given:

1. The results of the central and the ultimate fuzzy reliability of the proposed method has an only small relative difference in comparison with that of fuzzy reliability of the method [10]. In the all of the examples, the central fuzzy reliability FRc of the proposed method produces results closed to the results of the method [10]. It is found that the fuzzy reliability of the method [10] is the average of the reliabilities at the  $\alpha$ -cuts, which is similar to the notion of the central fuzzy reliability of the proposed method.
2. The results of the ultimate fuzzy reliability of the proposed method has an only small relative difference in comparison with that of fuzzy reliability of the method [6]. The relative differences have been occurred due to the different meaning between the fuzzy reliability in the method [6] and the reliability in the classical reliability theory.
3. The result of the ultimate fuzzy reliability of the proposed method approximates that of the reliability based on the classical approach. In addition, the proposed method need'n any assumption for distribution of uncertainty variables when information

deficit appears. This pointed out that the proposed method is suitable to handle the problems of reality.

## CONCLUSIONS

A novel method for calculating structural fuzzy reliability is proposed in this study. In order to apply the traditional reliability theory, a family of normal random variables equalling symmetric triangular fuzzy number are presented and explored in detail. From these equivalent random ones, two novel notions of the central fuzzy reliability and the ultimate fuzzy reliability are formulated and a procedure for calculating them is defined. In order to handle the bias of fuzzy reliability, the proposed ultimate reliability need to be compared with the allowable reliability in the structural design codes. Numerical results manifest the equivalence between the ultimate reliability in the proposed method and the "exact" reliability in the classical reliability theory.

## REFERENCES

1. Kiureghian, A.D., Ditlevsen, O. (2009). Aleatory or epistemic? Does it matter?. *Structural Safety*, vol. 15, no.2, 105-112. DOI: 10.1016/j.strusafe.2008.06.020
2. Nowak, A., Collins, K.R. (2012). *Reliability of Structures*. CRC Press
3. Melchers, R.E. (1999). *Structural Reliability Analysis and Prediction*. John Wiley & Sons.
4. ISO 2394: 2015 General principles on reliability for structures.
5. Dong, W., Chiang, W.L., Shan, H.C., Wong, F.S. (1989). Probabilistic Assessment of existing building using fuzzy set theory. *Icosar'89, The 5th International Conference on Structural Safety and Reliability*.
6. Sherstha, B., Duckstein, L. (1998). A fuzzy reliability measure for engineering applications. In: Ayyub, B.M. (Ed.), *Uncertainty Modelling and Analysis in Civil Engineering*. CRC Press, Boca Raton, p.120-135.
7. Park, H.J., Um, J.G., Woo, I., Kim, J.W. (2012). Application of fuzzy set theory to evaluate the probability of failure in rock slopes. *Engineering Geology*, vol. 125, no. 27, 92-101. DOI: 10.1016/j.enggeo.2011.11.008.
8. Rezaei, M., Fazlzadeh, S.A., Mazidi, A., Friswell, M.I., Khodaparast, H.H. (2020). Fuzzy uncertainty analysis and reliability assessment of aeroelastic aircraft wings. *The Aeronautical Journal*, vol. 124, 786-811. DOI: 10.1017/aer.2020.2
9. Li, B., Zhu, M., Xu, K. (2000). A practical engineering method for fuzzy reliability analysis of mechanical structures. *Reliability Engineering and System Safety*, vol. 67, no.3, 311-315. DOI: 10.1016/S0951-8320(99)00073-3.
10. Jiang, Q., Chen, C.H. (2003). A numerical algorithm of fuzzy reliability. *Reliability Engineering and System Safety*, vol. 80, no. 3, 299-307. DOI: 10.1016/S0951-8320(03)00055-3

11. Tang, Z., Lu, Z., Xia, Y. (2013). Numerical method for fuzzy reliability analysis. *Journal of Aircraft*, vol. 50, no. 6, 1710-1714. DOI: 10.2514/1.C031817.
12. Pan, C-Y., Wei, W-L., Zang, C-Y., Song, L-K., Lu, C., Liu, L-J. (2015). Reliability analysis of turbine blades based on fuzzy response surface method. *Journal of Intelligent & Fuzzy Systems*, vol. 29, 2467-2474. DOI: 10.3233/IFS-151947.
13. Möller, B., Beer, M. (2004). *Fuzzy Randomness - Uncertainty in Civil Engineering and Computational Mechanics*. Springer, Dresden.
14. Liu, Y., Qiao, Z., Wang, G. (1997). Fuzzy random reliability based on fuzzy random variable. *Fuzzy Sets and Systems*, vol. 86, no. 3, 345-355. DOI: 10.1016/S0165-0114(96)00002-4.
15. Klir, G.J. (2006). *Uncertainty and Information*. John Wiley & Sons.
16. DeGroot, H.M. (1989). *Probability and Statistics*. Addison-Wesley Publishing Company, New York.
17. Zadeh, L.A. (1999). Fuzzy sets as a basis for a theory of possibility. *Fuzzy sets and systems*, vol. 100, no. 1, 9-34. DOI: 10.1016/S0165-0114(99)80004-9.
18. Dubois, D., Foulloy, L., Mauris, G., Prade, H. (2004). Probability - Possibility Transformations, Triangular Fuzzy Sets, and Probabilistic Inequalities. *Reliable Computing*, vol. 10, 273-297. DOI: 10.1023/B:REOM.0000032115.22510
19. Smets P. (1990). Constructing the pignistic probability function in a context of uncertainty. *Machine Intelligence and Pattern Recognition*, vol.10, 29-39. DOI: 10.1016/B978-0-444-88738-2.50010-5.
20. Dubois D., Prade H., Sandri S. (1993). On Possibility/Probability Transformations. *Proceedings of Fourth IFSA Conference*: 103-112.
21. Dubois D. (2006). Possibility Theory and Statistical Reasoning. *Computational Statistics & Data Analysis*, vol. 51, no. 1, 47-69. DOI: 10.1016/j.csda.2006.04.015.
22. Tuan Hung Nguyen, Huynh Xuan Le (2019). A practical method for calculating reliability with a mixture of random and fuzzy variables. *Structural Integrity and Life*, vol.19, no. 3, 175-183.
23. Michalewics, Z. (1995). *Genetic Algorithms + Data Structures = Evolution Programs*. Springer.
24. Lei, Z., Chen, Q. (2002). A new approach to fuzzy finite element analysis. *Computer methods in applied mechanics and engineering*, vol.191, 5113-5118. DOI: 10.1016/S0045-7825(02)00240-2.
25. Mason, R.L., Guns, R.F., Hess, J.L. (2003). *Statistical Design and Analysis of Experiment : With Applications to Engineering and Science*. John Wiley & Sons.
26. Trinh Tho Leu, Binh Van Do (2006). *Structural stability*. Construction Publishing House (in Vietnamese).
27. Zao, Y.G., Ono, T. (1999). New Approximations for SORM: Part 1. *Journal of Engineering Mechanics*, vol. 85, 79-85. DOI: 10.1061/(ASCE)0733-9399(1999)125:1(79).
28. Zao, Y.G., Ono, T. (1999). New Approximations for SORM: Part 2. *Journal of Engineering Mechanics*, vol. 93, 86-93. DOI: 10.1061/(ASCE)0733-9399(1999)125:1(86).
29. Vietnamese national standard TCVN 9393:2012 Pile- Standard test method in situ for piles under axial compressive load (in Vietnamese).

*Paper submitted: 01.02.2021.*

*Paper accepted: 10.04.2021.*

*This is an open access article distributed under the  
CC BY 4.0 terms and conditions.*